

Design Handbook



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TECHNICAL DATA

PANEL SPECIFICATIONS

| Dimensions & Standard SIP Panel Properties | | | | | |
|--|-----------------------|------------------------|------------------------|-------------------------|-------------------------|
| SIP Thickness: inch (mm) | 4.5" (114) | 6.5" (165) | 8.25" (210) | 10.25" (260) | 12.25" (311) |
| EPS Core Thickness: inch (mm) | 3.625" (92) | 5.625" (143) | 7.375" (188) | 9.375" (238) | 11.375" (289) |
| Dimensional Lumber: nominal inches (actual mm) | 2x4 (38x89) | 2x6 (38x140) | 2x8 (38x185) | 2x10 (38x235) | 2x12 (38x285) |
| Weight: psf (kg/m ²) | 3.2 (15.6) | 3.4 (16.6) | 3.6 (17.6) | 3.8 (18.5) | 4.0 (19.5) |
| R-Value Performance ¹ (Prescriptive ²) | 24 (17.0) | 48 (25.8) | 55 (33.5) | 70 (42.3) | 85 (51.1) |

Design Handbook General Notes:

1. Thermal resistance compliance with Division A, 1.2.1.1.(1),(b) of the NBC.
2. EPS @ 25F ASTM C518; Structural Board Association's "OSB in Wood Frame Construction," (U.S. Edition 1996/97); compliance with Division A, 1.2.1.1.(1),(a) of the NBC.

LOAD COMBINATION

Designer to use ASD load combination to determine SIP panels capacity. The load combinations are shown as below:

| Case | Load Combination | |
|------|---|---|
| 1 | D | Where: D= Dead Load L = Live Load S= Snow Load Lr= Roof Live Load R = Rain Load W= Wind Load E = Earthquake Load |
| 2 | D+L | |
| 3 | D + (L _r or S or R) | |
| 4 | D + 0.75L + 0.75(L _r or S or R) | |
| 5 | D + (0.6W or 0.7E) | |
| 6a | D + 0.75(L + 0.6W + (L _r or S or R)) | |
| 6b | D + 0.75(L + 0.7E + S) | |
| 7 | 0.6D + 0.6W | |
| 8 | 0.6D + 0.7E | |

COMMON ABBREVIATIONS USED IN THIS HANDBOOK

plf: Pounds per linear foot (lb/ft)

psf: Pounds per square foot (lb/ft²)

SIP: Structural Insulated Panel

PWF: Permanent Wood Foundation

FULL LENGTH SIP PANELS with SIP SPLINES

TABLE 1

| Transverse Allowable Design Load (PSF) | | | | | |
|--|------------------|-----------------|----|----|----|
| Spline Type: SIP | | | | | |
| SIP Thickness (inches) | Deflection Limit | SIP Span (feet) | | | |
| | | 4 | 8 | 10 | 12 |
| 4.5 | L/360 | 76 | 38 | 24 | |
| | L/240 | 110 | 46 | 24 | |
| | L/180 | 142 | 46 | 24 | |
| 6.5 | L/360 | 145 | 61 | 35 | 25 |
| | L/240 | 154 | 61 | 51 | 39 |
| | L/180 | 154 | 61 | 51 | 41 |
| 8.25 | L/360 | 145 | 64 | 51 | 42 |
| | L/240 | 154 | 69 | 55 | 46 |
| | L/180 | 154 | 69 | 55 | 46 |
| 10.25 | L/360 | 145 | 65 | 52 | 43 |
| | L/240 | 154 | 74 | 59 | 49 |
| | L/180 | 154 | 74 | 59 | 49 |

General Notes:

1. Table values are for uniform area load pounds per square foot (psf).
2. Continuous support with a minimum bearing of 3" at each end.
3. Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
4. For sloped SIPs, the loading conditions and SIP capacities should be reviewed based upon the inclined SIP length.
5. Values are for the total load as per ASD load combination.
6. Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.

FULL LENGTH SIP PANEL with 2-PLY LUMBER SPLINE

TABLE 2

| Transverse Allowable Design Load (PSF) | | | | |
|---|------------------|---------------------|-----|-----|
| Spline Type: 2-Ply Lumber Spline NLGA SPF No. 2 or Better Grade | | | | |
| SIP Thickness (inches) | Deflection Limit | Panel Length (feet) | | |
| | | 8 | 10 | 12 |
| 4.5 | L/480 | 38 | 27 | 19 |
| | L/360 | 51 | 37 | 26 |
| | L/240 | 76 | 56 | 39 |
| | L/180 | 91 | 74 | 52 |
| 6.5 | L/480 | 83 | 50 | 38 |
| | L/360 | 110 | 68 | 51 |
| | L/240 | 164 | 106 | 77 |
| | L/180 | 164 | 121 | 80 |
| 8.25 | L/480 | 94 | 75 | 63 |
| | L/360 | 128 | 102 | 85 |
| | L/240 | 178 | 142 | 119 |
| | L/180 | 178 | 142 | 119 |
| 10.25 | L/480 | 106 | 85 | 71 |
| | L/360 | 142 | 113 | 94 |
| | L/240 | 191 | 153 | 127 |
| | L/180 | 196 | 153 | 127 |
| 12.25 | L/480 | 106 | 85 | 71 |
| | L/360 | 142 | 113 | 94 |
| | L/240 | 191 | 153 | 127 |
| | L/180 | 191 | 153 | 127 |

General Notes:

1. Table values are for uniform area load pounds per square foot (psf).
2. Continuous support with a minimum bearing of 3" at each end.
3. Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
4. For sloped SIPs, the loading conditions and SIP capacities should be reviewed based upon the inclined SIP length.
5. Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.
6. Lumber spline every 4ft apart.

4-FOOT SIP PANEL with 2-PLY LUMBER SPLINE

TABLE 3A

| Transverse Allowable Design Load (PSF) | | | | | | | | | | | |
|---|------------------|-----------------------------|-----|----|----|----|----|----|----|----|----|
| Spline Type: 2-Ply Lumber Spline NLGA SPF No. 1 / No. 2 or Better Grade | | | | | | | | | | | |
| SIP Thickness (inches) | Deflection Limit | Lumber Spline Length (feet) | | | | | | | | | |
| | | 4 | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 |
| 6.5 | L/480 | 94 | 31 | 16 | | | | | | | |
| | L/360 | 124 | 42 | 21 | 12 | | | | | | |
| | L/240 | 125 | 44 | 28 | 18 | 11 | | | | | |
| | L/180 | 125 | 44 | 28 | 19 | 14 | 10 | | | | |
| 8.25 | L/480 | 82 | 71 | 37 | 21 | 13 | | | | | |
| | L/360 | 109 | 71 | 45 | 28 | 18 | 12 | | | | |
| | L/240 | 134 | 71 | 45 | 31 | 23 | 17 | 12 | | | |
| | L/180 | 134 | 71 | 45 | 31 | 23 | 17 | 14 | 11 | | |
| 10.25 | L/480 | 97 | 97 | 68 | 44 | 28 | 18 | 13 | | | |
| | L/360 | 128 | 107 | 68 | 47 | 35 | 25 | 17 | 12 | | |
| | L/240 | 144 | 107 | 68 | 47 | 35 | 26 | 21 | 17 | 14 | 11 |
| | L/180 | 144 | 107 | 68 | 47 | 35 | 26 | 21 | 17 | 14 | 11 |
| 12.25 | L/480 | 97 | 97 | 92 | 64 | 47 | 33 | 23 | 17 | 13 | 10 |
| | L/360 | 128 | 128 | 92 | 64 | 47 | 36 | 28 | 23 | 17 | 13 |
| | L/240 | 144 | 144 | 92 | 64 | 47 | 36 | 28 | 23 | 19 | 16 |
| | L/180 | 144 | 144 | 92 | 64 | 47 | 36 | 28 | 23 | 19 | 16 |

General Notes:

1. Table values are for uniform area load pounds per square foot (psf) for SIP panels spanned 4ft between 2-Ply lumber spline.
2. Continuous support with a minimum bearing of 3" at each end.
3. Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
4. For sloped SIPs, the loading conditions and SIP capacities should be reviewed based upon the inclined SIP length.
5. Values are for the total load as per ASD load combination.
6. Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.
7. Lumber spline capacity based on NDS2015 allowable stress design.

4-FOOT SIP PANEL with 3-PLY LUMBER SPLINE

TABLE 3B

| Transverse Allowable Design Load (PSF) | | | | | | | | | | | |
|---|------------------|-----------------------------|-----|-----|----|----|----|----|----|----|----|
| Spline Type: 3-Ply Lumber Spline NLGA SPF No. 1 / No. 2 or Better Grade | | | | | | | | | | | |
| SIP Thickness (inches) | Deflection Limit | Lumber Spline Length (feet) | | | | | | | | | |
| | | 4 | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 |
| 6.5 | L/480 | 94 | 47 | 24 | | | | | | | |
| | L/360 | 124 | 63 | 32 | 18 | | | | | | |
| | L/240 | 125 | 67 | 43 | 28 | 17 | | | | | |
| | L/180 | 125 | 67 | 43 | 29 | 21 | 15 | | | | |
| 8.25 | L/480 | 82 | 82 | 55 | 32 | 20 | | | | | |
| | L/360 | 109 | 107 | 68 | 42 | 27 | 18 | | | | |
| | L/240 | 134 | 107 | 68 | 47 | 35 | 26 | 19 | | | |
| | L/180 | 134 | 107 | 68 | 47 | 35 | 26 | 21 | 17 | | |
| 10.25 | L/480 | 97 | 97 | 97 | 66 | 42 | 28 | 19 | | | |
| | L/360 | 128 | 128 | 102 | 71 | 52 | 37 | 26 | 19 | | |
| | L/240 | 144 | 144 | 102 | 71 | 52 | 40 | 31 | 25 | 21 | 16 |
| | L/180 | 144 | 144 | 102 | 71 | 52 | 40 | 31 | 25 | 21 | 17 |
| 12.25 | L/480 | 97 | 97 | 97 | 96 | 70 | 50 | 35 | 25 | 19 | 15 |
| | L/360 | 128 | 128 | 128 | 96 | 70 | 54 | 42 | 34 | 26 | 20 |
| | L/240 | 144 | 144 | 138 | 96 | 70 | 54 | 42 | 34 | 28 | 24 |
| | L/180 | 144 | 144 | 138 | 96 | 70 | 54 | 42 | 34 | 28 | 24 |

General Notes:

1. Table values are for uniform area load pounds per square foot (psf) for SIP panels spanned 4ft between 3-Ply lumber spline.
2. Continuous support with a minimum bearing of 3" at each end required.
3. Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
4. For sloped SIPs, the loading condition and SIP capacities should be reviewed based on the inclined SIP length.
5. Values are for the total load as per ASD load combinations.
6. Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.
7. Lumber spline capacity based on NDS2015 allowable stress design.

4-FOOT SIP PANEL with 4-PLY LUMBER SPLINE

TABLE 3C

| Transverse Allowable Design Load (PSF) | | | | | | | | | | | |
|---|------------------|-----------------------------|-----|-----|-----|-----|----|----|----|----|----|
| Spline Type: 4-Ply Lumber Spline NLGA SPF No. 1 / No. 2 or Better Grade | | | | | | | | | | | |
| SIP Thickness (inches) | Deflection Limit | Lumber Spline Length (feet) | | | | | | | | | |
| | | 4 | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 |
| 6.5 | L/480 | 94 | 63 | 32 | | | | | | | |
| | L/360 | 124 | 84 | 43 | 24 | | | | | | |
| | L/240 | 125 | 103 | 64 | 37 | 23 | | | | | |
| | L/180 | 125 | 103 | 65 | 45 | 31 | 21 | | | | |
| 8.25 | L/480 | 82 | 82 | 74 | 42 | 27 | | | | | |
| | L/360 | 109 | 109 | 98 | 57 | 36 | 24 | | | | |
| | L/240 | 134 | 134 | 105 | 73 | 53 | 36 | 25 | | | |
| | L/180 | 134 | 134 | 105 | 73 | 53 | 41 | 32 | 24 | | |
| 10.25 | L/480 | 97 | 97 | 97 | 89 | 56 | 37 | 26 | | | |
| | L/360 | 128 | 128 | 128 | 109 | 74 | 50 | 35 | 25 | | |
| | L/240 | 144 | 144 | 144 | 109 | 80 | 61 | 48 | 38 | 28 | 22 |
| | L/180 | 144 | 144 | 144 | 109 | 80 | 61 | 48 | 39 | 32 | 27 |
| 12.25 | L/480 | 97 | 97 | 97 | 97 | 97 | 67 | 47 | 34 | 26 | 20 |
| | L/360 | 128 | 128 | 128 | 128 | 108 | 82 | 63 | 46 | 34 | 26 |
| | L/240 | 144 | 144 | 144 | 144 | 108 | 82 | 65 | 53 | 43 | 36 |
| | L/180 | 144 | 144 | 144 | 144 | 108 | 82 | 65 | 53 | 43 | 36 |

General Notes:

1. Table values are for uniform area load pounds per square foot (psf) for SIP panels spanned 4ft between 3-Ply lumber spline.
2. Continuous support with a minimum bearing of 3" at each end required.
3. Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
4. For sloped SIPs, the loading condition and SIP capacities should be reviewed based on the inclined SIP length.
5. Values are for the total load as per ASD load combinations.
6. Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.
7. Lumber spline capacity based on NDS2015 allowable stress design.

4-FOOT SIP PANEL with 2-PLY LSL SPLINES

TABLE 4A

| Transverse Allowable Design Load (PSF) | | | | | | | | | | | | | | | |
|--|--------------------|------------------|-----------------------------|-----|-----|-----|-----|-----|----|----|----|----|----|----|----|
| Spline Type: 2-Ply LSL Spline | | | | | | | | | | | | | | | |
| SIP Thickness (inches) | LVL Depth (inches) | Deflection Limit | Lumber Spline Length (feet) | | | | | | | | | | | | |
| | | | 4 | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 | 26 | 28 | 30 |
| 4.5 | 3.5 | L/480 | 28 | | | | | | | | | | | | |
| | | L/360 | 38 | 13 | | | | | | | | | | | |
| | | L/240 | 46 | 20 | | | | | | | | | | | |
| | | L/180 | 46 | 27 | 14 | | | | | | | | | | |
| 6.5 | 5.5 | L/480 | 94 | 38 | 20 | 11 | | | | | | | | | |
| | | L/360 | 124 | 51 | 26 | 15 | | | | | | | | | |
| | | L/240 | 125 | 77 | 40 | 23 | 14 | | | | | | | | |
| | | L/180 | 125 | 103 | 53 | 31 | 19 | 13 | | | | | | | |
| 8.25 | 7.25 | L/480 | 82 | 82 | 45 | 26 | 16 | | | | | | | | |
| | | L/360 | 109 | 109 | 60 | 35 | 22 | 15 | | | | | | | |
| | | L/240 | 134 | 134 | 90 | 53 | 33 | 22 | 16 | 11 | | | | | |
| | | L/180 | 134 | 134 | 120 | 71 | 45 | 30 | 21 | 15 | 11 | | | | |
| 10.25 | 9.25 | L/480 | 97 | 97 | 91 | 54 | 34 | 23 | 16 | 12 | | | | | |
| | | L/360 | 128 | 128 | 121 | 72 | 46 | 31 | 22 | 16 | 12 | | | | |
| | | L/240 | 144 | 144 | 144 | 108 | 69 | 46 | 33 | 24 | 18 | 14 | | | |
| | | L/180 | 144 | 144 | 144 | 139 | 92 | 62 | 44 | 32 | 24 | 18 | 14 | 11 | |
| 12.25 | 11.25 | L/480 | 97 | 97 | 97 | 94 | 60 | 41 | 29 | 21 | 16 | 12 | | | |
| | | L/360 | 128 | 128 | 128 | 126 | 81 | 55 | 39 | 28 | 21 | 16 | 13 | | |
| | | L/240 | 144 | 144 | 144 | 144 | 121 | 82 | 58 | 43 | 32 | 25 | 19 | 16 | 13 |
| | | L/180 | 144 | 144 | 144 | 144 | 144 | 110 | 78 | 57 | 43 | 33 | 26 | 21 | 17 |

General Notes:

- Table values are for uniform area load pounds per square foot (psf), for SIP panels spanned 4ft between 2-Ply LSL spline.
- Continuous support with a minimum bearing of 3" at each end required.
- Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
- For sloped SIPs, the loading conditions and SIP capacities should be reviewed based on the inclined SIP length.
- Values are for the total load as per ASD load combination.
- Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.
- Deflection values include shear deformation of LSL beam.
- LSL properties shall not be less than:
 $F_b(\text{psi}): 2325$ $F_v(\text{psi}): 310$ $E(\text{psi}): 1.55E+06$
- Table values are based on each LSL ply to be 1-3/4" thick.

4-FOOT SIP PANEL with 3-PLY LSL SPLINES

TABLE 4B

| Transverse Allowable Design Load (PSF) | | | | | | | | | | | | | | | |
|--|--------------------|------------------|-----------------------------|-----|-----|-----|-----|-----|-----|----|----|----|----|----|----|
| Spline Type: 3-Ply LSL Spline | | | | | | | | | | | | | | | |
| SIP Thickness (inches) | LVL Depth (inches) | Deflection Limit | Lumber Spline Length (feet) | | | | | | | | | | | | |
| | | | 4 | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 | 26 | 28 | 30 |
| 4.5 | 3.5 | L/480 | 28 | | | | | | | | | | | | |
| | | L/360 | 38 | 20 | | | | | | | | | | | |
| | | L/240 | 46 | 30 | | | | | | | | | | | |
| | | L/180 | 46 | 41 | 21 | | | | | | | | | | |
| 6.5 | 5.5 | L/480 | 94 | 58 | 30 | 17 | | | | | | | | | |
| | | L/360 | 124 | 77 | 40 | 23 | | | | | | | | | |
| | | L/240 | 125 | 116 | 60 | 35 | 22 | | | | | | | | |
| | | L/180 | 125 | 125 | 80 | 47 | 29 | 20 | | | | | | | |
| 8.25 | 7.25 | L/480 | 82 | 82 | 67 | 39 | 25 | | | | | | | | |
| | | L/360 | 109 | 109 | 90 | 53 | 33 | 22 | | | | | | | |
| | | L/240 | 134 | 134 | 134 | 79 | 50 | 34 | 24 | 17 | | | | | |
| | | L/180 | 134 | 134 | 134 | 106 | 67 | 45 | 32 | 23 | 17 | | | | |
| 10.25 | 9.25 | L/480 | 97 | 97 | 97 | 81 | 51 | 35 | 24 | 18 | | | | | |
| | | L/360 | 128 | 128 | 128 | 108 | 69 | 46 | 33 | 24 | 18 | | | | |
| | | L/240 | 144 | 144 | 144 | 144 | 103 | 70 | 49 | 36 | 27 | 21 | | | |
| | | L/180 | 144 | 144 | 144 | 144 | 138 | 93 | 66 | 48 | 36 | 28 | 22 | 17 | |
| 12.25 | 11.25 | L/480 | 97 | 97 | 97 | 97 | 91 | 62 | 44 | 32 | 24 | 18 | | | |
| | | L/360 | 128 | 128 | 128 | 128 | 121 | 82 | 58 | 43 | 32 | 25 | 19 | | |
| | | L/240 | 144 | 144 | 144 | 144 | 144 | 124 | 88 | 64 | 49 | 37 | 29 | 24 | 19 |
| | | L/180 | 144 | 144 | 144 | 144 | 144 | 144 | 117 | 86 | 65 | 50 | 39 | 32 | 26 |

General Notes:

1. Table values are for uniform area load pounds per square foot (psf), for SIP panels spanned 4ft between 3-ply LSL spline.
2. Continuous support with a minimum bearing of 3" at each end required.
3. Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
4. For sloped SIPs, the loading conditions and SIP capacities should be reviewed based on the inclined SIP length.
5. Values are for the total load as per ASD load combination.
6. Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.
7. Deflection values include shear deformation of LSL beam.
8. LSL properties shall not be less than:
 Fb(psi): 2325 Fv(psi): 310 E (psi): 1.55E+06
9. Table values are based on each LSL ply to be 1-3/4" thick.

4-FOOT SIP PANEL with 4-PLY LSL SPLINES

TABLE 4C

| Transverse Allowable Design Load (PSF) | | | | | | | | | | | | | | | |
|--|--------------------|------------------|-----------------------------|-----|-----|-----|-----|-----|-----|-----|----|----|----|----|----|
| Spline Type: 4-Ply LSL Spline | | | | | | | | | | | | | | | |
| SIP Thickness (inches) | LVL Depth (inches) | Deflection Limit | Lumber Spline Length (feet) | | | | | | | | | | | | |
| | | | 4 | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 | 26 | 28 | 30 |
| 4.5 | 3.5 | L/480 | 28 | | | | | | | | | | | | |
| | | L/360 | 38 | 27 | | | | | | | | | | | |
| | | L/240 | 46 | 41 | | | | | | | | | | | |
| | | L/180 | 46 | 46 | 28 | | | | | | | | | | |
| 6.5 | 5.5 | L/480 | 94 | 77 | 40 | 23 | | | | | | | | | |
| | | L/360 | 124 | 103 | 53 | 31 | | | | | | | | | |
| | | L/240 | 125 | 125 | 80 | 47 | 29 | | | | | | | | |
| | | L/180 | 125 | 125 | 107 | 63 | 39 | 26 | | | | | | | |
| 8.25 | 7.25 | L/480 | 82 | 82 | 82 | 53 | 33 | | | | | | | | |
| | | L/360 | 109 | 109 | 109 | 71 | 45 | 30 | | | | | | | |
| | | L/240 | 134 | 134 | 134 | 106 | 67 | 45 | 32 | 23 | | | | | |
| | | L/180 | 134 | 134 | 134 | 134 | 90 | 60 | 43 | 31 | 23 | | | | |
| 10.25 | 9.25 | L/480 | 97 | 97 | 97 | 97 | 69 | 46 | 33 | 24 | | | | | |
| | | L/360 | 128 | 128 | 128 | 128 | 92 | 62 | 44 | 32 | 24 | | | | |
| | | L/240 | 144 | 144 | 144 | 144 | 138 | 93 | 66 | 48 | 36 | 28 | | | |
| | | L/180 | 144 | 144 | 144 | 144 | 144 | 124 | 88 | 64 | 48 | 37 | 29 | 23 | |
| 12.25 | 11.25 | L/480 | 97 | 97 | 97 | 97 | 97 | 82 | 58 | 43 | 32 | 25 | | | |
| | | L/360 | 128 | 128 | 128 | 128 | 128 | 110 | 78 | 57 | 43 | 33 | 26 | | |
| | | L/240 | 144 | 144 | 144 | 144 | 144 | 144 | 117 | 86 | 65 | 50 | 39 | 32 | 26 |
| | | L/180 | 144 | 144 | 144 | 144 | 144 | 144 | 144 | 115 | 87 | 67 | 53 | 42 | 34 |

General Notes:

1. Table values are for uniform area load pounds per square foot (psf), for SIP panels spanned 4ft between 4-ply LSL spline.
2. Continuous support with a minimum bearing of 3" at each end required.
3. Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
4. For sloped SIPs, the loading conditions and SIP capacities should be reviewed based on the inclined SIP length.
5. Values are for the total load as per ASD load combination.
6. Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.
7. Deflection values include shear deformation of LSL beam.
8. LSL properties shall not be less than:
 Fb(psi): 2325 Fv(psi): 310 E (psi): 1.55E+06
9. Table values are based on each LSL ply to be 1-3/4" thick.

4-FOOT SIP PANEL with 2-PLY LVL SPLINES

TABLE 5A

| Transverse Allowable Design Load (PSF) | | | | | | | | | | | | | | | |
|--|--------------------|------------------|-----------------------------|-----|-----|-----|-----|-----|----|----|----|----|----|----|----|
| Spline Type: 2-Ply LVL Spline | | | | | | | | | | | | | | | |
| SIP Thickness (inches) | LVL Depth (inches) | Deflection Limit | Lumber Spline Length (feet) | | | | | | | | | | | | |
| | | | 4 | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 | 26 | 28 | 30 |
| 4.5 | 3.5 | L/480 | 28 | | | | | | | | | | | | |
| | | L/360 | 38 | 17 | | | | | | | | | | | |
| | | L/240 | 46 | 26 | | | | | | | | | | | |
| | | L/180 | 46 | 35 | 18 | | | | | | | | | | |
| 6.5 | 5.5 | L/480 | 94 | 50 | 26 | 15 | | | | | | | | | |
| | | L/360 | 124 | 66 | 34 | 20 | | | | | | | | | |
| | | L/240 | 125 | 100 | 52 | 30 | 19 | | | | | | | | |
| | | L/180 | 125 | 125 | 69 | 40 | 25 | 17 | | | | | | | |
| 8.25 | 7.25 | L/480 | 82 | 82 | 58 | 34 | 21 | | | | | | | | |
| | | L/360 | 109 | 109 | 77 | 45 | 29 | 19 | | | | | | | |
| | | L/240 | 134 | 134 | 116 | 68 | 43 | 29 | 20 | 15 | | | | | |
| | | L/180 | 134 | 134 | 134 | 91 | 58 | 39 | 27 | 20 | 15 | | | | |
| 10.25 | 9.25 | L/480 | 97 | 97 | 97 | 69 | 44 | 30 | 21 | 15 | | | | | |
| | | L/360 | 128 | 128 | 128 | 93 | 59 | 40 | 28 | 20 | 15 | | | | |
| | | L/240 | 144 | 144 | 144 | 139 | 89 | 60 | 42 | 31 | 23 | 18 | | | |
| | | L/180 | 144 | 144 | 144 | 144 | 114 | 80 | 57 | 41 | 31 | 24 | 19 | 15 | |
| 12.25 | 11.25 | L/480 | 97 | 97 | 97 | 97 | 78 | 53 | 37 | 27 | 21 | 16 | | | |
| | | L/360 | 128 | 128 | 128 | 128 | 104 | 71 | 50 | 37 | 28 | 21 | 17 | | |
| | | L/240 | 144 | 144 | 144 | 144 | 144 | 107 | 75 | 55 | 42 | 32 | 25 | 20 | 16 |
| | | L/180 | 144 | 144 | 144 | 144 | 144 | 126 | 99 | 74 | 56 | 43 | 34 | 27 | 22 |

General Notes:

- Table values are for uniform area load pounds per square foot (psf), for SIP panels spanned 4ft between 2-Ply LVL spline.
- Continuous support with a minimum bearing of 3" at each end required.
- Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
- For sloped SIPs, the loading conditions and SIP capacities should be reviewed based on the inclined SIP length.
- Values are for the total load as per ASD load combination.
- Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.
- Deflection values include shear deformation of LVL beam.
- LVL properties shall not be less than:
 Fb(psi): 2600 Fv(psi): 285 E (psi): 2.00E+06

4-FOOT SIP PANEL with 3-PLY LVL SPLINES

TABLE 5B

| Transverse Allowable Design Load (PSF) | | | | | | | | | | | | | | | |
|--|--------------------|------------------|-----------------------------|-----|-----|-----|-----|-----|-----|-----|----|----|----|----|----|
| Spline Type: 3-Ply LVL Spline | | | | | | | | | | | | | | | |
| SIP Thickness (inches) | LVL Depth (inches) | Deflection Limit | Lumber Spline Length (feet) | | | | | | | | | | | | |
| | | | 4 | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 | 26 | 28 | 30 |
| 4.5 | 3.5 | L/480 | 28 | | | | | | | | | | | | |
| | | L/360 | 38 | 26 | | | | | | | | | | | |
| | | L/240 | 46 | 39 | | | | | | | | | | | |
| | | L/180 | 46 | 46 | 27 | | | | | | | | | | |
| 6.5 | 5.5 | L/480 | 94 | 75 | 39 | 22 | | | | | | | | | |
| | | L/360 | 124 | 100 | 52 | 30 | | | | | | | | | |
| | | L/240 | 125 | 125 | 78 | 45 | 28 | | | | | | | | |
| | | L/180 | 125 | 125 | 104 | 61 | 38 | 25 | | | | | | | |
| 8.25 | 7.25 | L/480 | 82 | 82 | 82 | 51 | 32 | | | | | | | | |
| | | L/360 | 109 | 109 | 109 | 68 | 43 | 29 | | | | | | | |
| | | L/240 | 134 | 134 | 134 | 103 | 65 | 44 | 31 | 22 | | | | | |
| | | L/180 | 134 | 134 | 134 | 134 | 87 | 59 | 41 | 30 | 22 | | | | |
| 10.25 | 9.25 | L/480 | 97 | 97 | 97 | 97 | 66 | 45 | 32 | 23 | | | | | |
| | | L/360 | 128 | 128 | 128 | 128 | 89 | 60 | 42 | 31 | 23 | | | | |
| | | L/240 | 144 | 144 | 144 | 144 | 133 | 90 | 64 | 47 | 35 | 27 | | | |
| | | L/180 | 144 | 144 | 144 | 144 | 144 | 120 | 85 | 62 | 47 | 36 | 28 | 23 | |
| 12.25 | 11.25 | L/480 | 97 | 97 | 97 | 97 | 97 | 80 | 56 | 41 | 31 | 24 | | | |
| | | L/360 | 128 | 128 | 128 | 128 | 128 | 107 | 75 | 55 | 42 | 32 | 25 | | |
| | | L/240 | 144 | 144 | 144 | 144 | 144 | 144 | 113 | 83 | 63 | 48 | 38 | 30 | 25 |
| | | L/180 | 144 | 144 | 144 | 144 | 144 | 144 | 144 | 111 | 84 | 65 | 51 | 41 | 33 |

General Notes:

1. Table values are for uniform area load pounds per square foot (psf), for SIP panels spanned 4ft between 3-ply LVL spline.
2. Continuous support with a minimum bearing of 3" at each end required.
3. Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
4. For sloped SIPs, the loading conditions and SIP capacities should be reviewed based on the inclined SIP length.
5. Values are for the total load as per ASD load combination.
6. Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.
7. Deflection values include shear deformation of LVL beam.
8. LVL properties shall not be less than:
 Fb(psi): 2600 Fv(psi): 285 E (psi): 2.00E+06

4-FOOT SIP PANEL with 4-PLY LVL SPLINES

TABLE 5C

| Transverse Allowable Design Load (PSF) | | | | | | | | | | | | | | | |
|--|--------------------|------------------|-----------------------------|-----|-----|-----|-----|-----|-----|-----|-----|----|----|----|----|
| Spline Type: 4-Ply LVL Spline | | | | | | | | | | | | | | | |
| SIP Thickness (inches) | LVL Depth (inches) | Deflection Limit | Lumber Spline Length (feet) | | | | | | | | | | | | |
| | | | 4 | 8 | 10 | 12 | 14 | 16 | 18 | 20 | 22 | 24 | 26 | 28 | 30 |
| 4.5 | 3.5 | L/480 | 28 | | | | | | | | | | | | |
| | | L/360 | 38 | 35 | | | | | | | | | | | |
| | | L/240 | 46 | 46 | | | | | | | | | | | |
| | | L/180 | 46 | 46 | 36 | | | | | | | | | | |
| 6.5 | 5.5 | L/480 | 94 | 94 | 52 | 30 | | | | | | | | | |
| | | L/360 | 124 | 124 | 69 | 40 | | | | | | | | | |
| | | L/240 | 125 | 125 | 104 | 61 | 38 | | | | | | | | |
| | | L/180 | 125 | 125 | 125 | 81 | 51 | 34 | | | | | | | |
| 8.25 | 7.25 | L/480 | 82 | 82 | 82 | 68 | 43 | | | | | | | | |
| | | L/360 | 109 | 109 | 109 | 91 | 58 | 39 | | | | | | | |
| | | L/240 | 134 | 134 | 134 | 134 | 87 | 59 | 41 | 30 | | | | | |
| | | L/180 | 134 | 134 | 134 | 134 | 116 | 78 | 55 | 40 | 30 | | | | |
| 10.25 | 9.25 | L/480 | 97 | 97 | 97 | 97 | 89 | 60 | 42 | 31 | | | | | |
| | | L/360 | 128 | 128 | 128 | 128 | 119 | 80 | 57 | 41 | 31 | | | | |
| | | L/240 | 144 | 144 | 144 | 144 | 144 | 120 | 85 | 62 | 47 | 36 | | | |
| | | L/180 | 144 | 144 | 144 | 144 | 144 | 144 | 114 | 83 | 63 | 48 | 38 | 30 | |
| 12.25 | 11.25 | L/480 | 97 | 97 | 97 | 97 | 97 | 97 | 75 | 55 | 42 | 32 | | | |
| | | L/360 | 128 | 128 | 128 | 128 | 128 | 128 | 101 | 74 | 56 | 43 | 34 | | |
| | | L/240 | 144 | 144 | 144 | 144 | 144 | 144 | 144 | 111 | 84 | 65 | 51 | 41 | 33 |
| | | L/180 | 144 | 144 | 144 | 144 | 144 | 144 | 144 | 144 | 112 | 86 | 68 | 55 | 44 |

General Notes:

1. Table values are for uniform area load pounds per square foot (psf), for SIP panels spanned 4ft between 4-ply LVL spline.
2. Continuous support with a minimum bearing of 3" at each end required.
3. Loads limited by deflection or ultimate failure load divided by a Factor of Safety (FoS) of 3.
4. For sloped SIPs, the loading conditions and SIP capacities should be reviewed based on the inclined SIP length.
5. Values are for the total load as per ASD load combination.
6. Deflection based upon $K_{cr}=1.0$. For long term deflection under sustained load (creep), additional deflection must be evaluated.
7. Deflection values include shear deformation of LVL beam.
8. LVL properties shall not be less than:
 Fb(psi): 2600 Fv(psi): 285 E (psi): 2.00E+06

EXTERIOR WALL DESIGN

SIP WALL PANEL with SIP SPLINE

TABLE 6A

| Maximum Allowable Axial Load for 4.5" Panel (PLF) | | | | | | | | | | |
|---|------------------|-----------|------|------|------|------|------|------|-----|-----|
| Spline Type: SIP Spline | | | | | | | | | | |
| Panel Height (feet) | Deflection Limit | Wind Load | | | | | | | | |
| | | 0 | 5 | 10 | 15 | 20 | 25 | 30 | 35 | 40 |
| 8 | L/360 | 3229 | 2882 | 2535 | 2188 | 1840 | 1493 | 1146 | 799 | 451 |
| | L/240 | 3229 | 2882 | 2535 | 2188 | 1840 | 1493 | 1146 | 799 | 451 |
| | L/180 | 3229 | 2882 | 2535 | 2188 | 1840 | 1493 | 1146 | 799 | 451 |
| 9 | L/360 | 3319 | 2886 | 2453 | 2019 | 1586 | 1153 | 720 | 286 | |
| | L/240 | 3319 | 2886 | 2453 | 2019 | 1586 | 1153 | 720 | 286 | |
| | L/180 | 3319 | 2886 | 2453 | 2019 | 1586 | 1153 | 720 | 286 | |
| 10 | L/360 | 3158 | 2700 | 2242 | 1784 | 1326 | 868 | 410 | | |
| | L/240 | 3158 | 2700 | 2242 | 1784 | 1326 | 868 | 410 | | |
| | L/180 | 3158 | 2700 | 2242 | 1784 | 1326 | 868 | 410 | | |

TABLE 6B

| Maximum Allowable Axial Load for 6.5" Panel (PLF) | | | | | | | | | | |
|---|------------------|-----------|------|------|------|------|------|------|------|------|
| Spline Type: SIP Spline | | | | | | | | | | |
| Panel Height (feet) | Deflection Limit | Wind Load | | | | | | | | |
| | | 0 | 5 | 10 | 15 | 20 | 25 | 30 | 35 | 40 |
| 8 | L/360 | 4452 | 4090 | 3728 | 3366 | 3004 | 2642 | 2280 | 1918 | 1556 |
| | L/240 | 4452 | 4090 | 3728 | 3366 | 3004 | 2642 | 2280 | 1918 | 1556 |
| | L/180 | 4452 | 4090 | 3728 | 3366 | 3004 | 2642 | 2280 | 1918 | 1556 |
| 9 | L/360 | 4065 | 3658 | 3250 | 2842 | 2434 | 2026 | 1618 | 1211 | 803 |
| | L/240 | 4065 | 3658 | 3250 | 2842 | 2434 | 2026 | 1618 | 1211 | 803 |
| | L/180 | 4065 | 3658 | 3250 | 2842 | 2434 | 2026 | 1618 | 1211 | 803 |
| 10 | L/360 | 3227 | 2868 | 2508 | 2148 | 1788 | 1429 | 1069 | 709 | 350 |
| | L/240 | 3227 | 2868 | 2508 | 2148 | 1788 | 1429 | 1069 | 709 | 350 |
| | L/180 | 3227 | 2868 | 2508 | 2148 | 1788 | 1429 | 1069 | 709 | 350 |
| 12 | L/360 | 3965 | 3484 | 3003 | 2522 | 2042 | 1561 | 1080 | 600 | 119 |
| | L/240 | 3965 | 3484 | 3003 | 2522 | 2042 | 1561 | 1080 | 600 | 119 |
| | L/180 | 3965 | 3484 | 3003 | 2522 | 2042 | 1561 | 1080 | 600 | 119 |

General Notes:

1. Table values are for linear pound per foot (lb/ft).
2. Loads Limited by Deflection or Ultimate Failure Load.
3. Tables values are for concentrated axial load with no eccentricity.

EXTERIOR WALL DESIGN

SIP WALL PANEL with SIP SPLINE (continued)

TABLE 6C

| Maximum Allowable Axial Load for 8.25" Panel (PLF) | | | | | | | | | | |
|--|------------------|-----------|------|------|------|------|------|------|------|------|
| Spline Type: SIP Spline | | | | | | | | | | |
| Panel Height (feet) | Deflection Limit | Wind Load | | | | | | | | |
| | | 0 | 5 | 10 | 15 | 20 | 25 | 30 | 35 | 40 |
| 8 | L/360 | 4048 | 3757 | 3466 | 3175 | 2884 | 2594 | 2303 | 2012 | 1721 |
| | L/240 | 4048 | 3757 | 3466 | 3175 | 2884 | 2594 | 2303 | 2012 | 1721 |
| | L/180 | 4048 | 3757 | 3466 | 3175 | 2884 | 2594 | 2303 | 2012 | 1721 |
| 9 | L/360 | 4048 | 3721 | 3393 | 3066 | 2739 | 2412 | 2085 | 1757 | 1430 |
| | L/240 | 4048 | 3721 | 3393 | 3066 | 2739 | 2412 | 2085 | 1757 | 1430 |
| | L/180 | 4048 | 3721 | 3393 | 3066 | 2739 | 2412 | 2085 | 1757 | 1430 |
| 10 | L/360 | 4048 | 3684 | 3321 | 2957 | 2594 | 2230 | 1866 | 1503 | 1139 |
| | L/240 | 4048 | 3684 | 3321 | 2957 | 2594 | 2230 | 1866 | 1503 | 1139 |
| | L/180 | 4048 | 3684 | 3321 | 2957 | 2594 | 2230 | 1866 | 1503 | 1139 |
| 12 | L/360 | 4048 | 3612 | 3175 | 2739 | 2303 | 1866 | 1430 | 994 | 557 |
| | L/240 | 4048 | 3612 | 3175 | 2739 | 2303 | 1866 | 1430 | 994 | 557 |
| | L/180 | 4048 | 3612 | 3175 | 2739 | 2303 | 1866 | 1430 | 994 | 557 |

TABLE 6D

| Maximum Allowable Axial Load for 10.25" Panel (PLF) | | | | | | | | | | |
|---|------------------|-----------|------|------|------|------|------|------|------|------|
| Spline Type: SIP Spline | | | | | | | | | | |
| Panel Height (feet) | Deflection Limit | Wind Load | | | | | | | | |
| | | 0 | 5 | 10 | 15 | 20 | 25 | 30 | 35 | 40 |
| 8 | L/360 | 4048 | 3775 | 3502 | 3229 | 2956 | 2683 | 2410 | 2137 | 1864 |
| | L/240 | 4048 | 3775 | 3502 | 3229 | 2956 | 2683 | 2410 | 2137 | 1864 |
| | L/180 | 4048 | 3775 | 3502 | 3229 | 2956 | 2683 | 2410 | 2137 | 1864 |
| 9 | L/360 | 4048 | 3741 | 3433 | 3126 | 2819 | 2512 | 2205 | 1898 | 1590 |
| | L/240 | 4048 | 3741 | 3433 | 3126 | 2819 | 2512 | 2205 | 1898 | 1590 |
| | L/180 | 4048 | 3741 | 3433 | 3126 | 2819 | 2512 | 2205 | 1898 | 1590 |
| 10 | L/360 | 4048 | 3707 | 3365 | 3024 | 2683 | 2341 | 2000 | 1659 | 1317 |
| | L/240 | 4048 | 3707 | 3365 | 3024 | 2683 | 2341 | 2000 | 1659 | 1317 |
| | L/180 | 4048 | 3707 | 3365 | 3024 | 2683 | 2341 | 2000 | 1659 | 1317 |
| 12 | L/360 | 4048 | 3638 | 3229 | 2819 | 2410 | 2000 | 1590 | 1181 | 771 |
| | L/240 | 4048 | 3638 | 3229 | 2819 | 2410 | 2000 | 1590 | 1181 | 771 |
| | L/180 | 4048 | 3638 | 3229 | 2819 | 2410 | 2000 | 1590 | 1181 | 771 |

General Notes:

1. Table values are for linear pound per foot (lb/ft).
2. Loads Limited by Deflection or Ultimate Failure Load.
3. Tables values are for concentrated axial load with no eccentricity.

EXTERIOR WALL DESIGN

SIP WALL PANEL with 2-PLY LUMBER SPLINE

TABLE 7A

| Maximum Allowable Axial Load for 4.5" Panel (PLF) | | | | | | | | | | |
|---|------------------|-----------|------|------|------|------|------|------|------|------|
| Spline Type: 2-Ply Lumber Spline | | | | | | | | | | |
| Panel Height (feet) | Deflection Limit | Wind Load | | | | | | | | |
| | | 0 | 5 | 10 | 15 | 20 | 25 | 30 | 35 | 40 |
| 8 | L/360 | 3229 | 3054 | 2878 | 2702 | 2526 | 2350 | 2174 | 1308 | 0 |
| | L/240 | 3229 | 3054 | 2878 | 2702 | 2526 | 2350 | 2174 | 1998 | 1822 |
| | L/180 | 3229 | 3054 | 2878 | 2702 | 2526 | 2350 | 2174 | 1998 | 1822 |
| 9 | L/360 | 3319 | 3128 | 2936 | 2744 | 2553 | 1339 | | | |
| | L/240 | 3319 | 3128 | 2936 | 2744 | 2553 | 2361 | 2170 | 1978 | 1786 |
| | L/180 | 3319 | 3128 | 2936 | 2744 | 2553 | 2361 | 2170 | 1978 | 1786 |
| 10 | L/360 | 3158 | 2954 | 2749 | 2544 | 2212 | | | | |
| | L/240 | 3158 | 2954 | 2749 | 2544 | 2339 | 2135 | 1930 | 1725 | 1520 |
| | L/180 | 3158 | 2954 | 2749 | 2544 | 2339 | 2135 | 1930 | 1725 | 1520 |
| 12 | L/360 | 1935 | 1581 | 1226 | 871 | 523 | | | | |
| | L/240 | 2315 | 2110 | 1904 | 1699 | 1491 | 1146 | 801 | 452 | 94 |
| | L/180 | 2315 | 2110 | 1904 | 1699 | 1493 | 1287 | 1082 | 876 | 671 |

TABLE 7B

| Maximum Allowable Axial Load for 6.5" Panel (PLF) | | | | | | | | | | |
|---|------------------|-----------|------|------|------|------|------|------|------|------|
| Spline Type: 2-Ply Lumber Spline | | | | | | | | | | |
| Panel Height (feet) | Deflection Limit | Wind Load | | | | | | | | |
| | | 0 | 5 | 10 | 15 | 20 | 25 | 30 | 35 | 40 |
| 8 | L/360 | 3334 | 3233 | 3131 | 3030 | 2929 | 2827 | 2726 | 2625 | 2523 |
| | L/240 | 3334 | 3233 | 3131 | 3030 | 2929 | 2827 | 2726 | 2625 | 2523 |
| | L/180 | 3334 | 3233 | 3131 | 3030 | 2929 | 2827 | 2726 | 2625 | 2523 |
| 9 | L/360 | 3531 | 3400 | 3269 | 3139 | 3008 | 2878 | 2747 | 2617 | 2486 |
| | L/240 | 3531 | 3400 | 3269 | 3139 | 3008 | 2878 | 2747 | 2617 | 2486 |
| | L/180 | 3531 | 3400 | 3269 | 3139 | 3008 | 2878 | 2747 | 2617 | 2486 |
| 10 | L/360 | 3938 | 3776 | 3614 | 3453 | 3291 | 3129 | 2967 | 2806 | 2644 |
| | L/240 | 3938 | 3776 | 3614 | 3453 | 3291 | 3129 | 2967 | 2806 | 2644 |
| | L/180 | 3938 | 3776 | 3614 | 3453 | 3291 | 3129 | 2967 | 2806 | 2644 |
| 12 | L/360 | 3965 | 3719 | 3474 | 3229 | 2984 | 2738 | 2493 | 2248 | 2003 |
| | L/240 | 3965 | 3719 | 3474 | 3229 | 2984 | 2738 | 2493 | 2248 | 2003 |
| | L/180 | 3965 | 3719 | 3474 | 3229 | 2984 | 2738 | 2493 | 2248 | 2003 |

General Notes:

1. Table values are for linear pound per foot (lb/ft).
2. Loads Limited by Deflection or Ultimate Failure Load.
3. Tables values are for concentrated axial load with no eccentricity.
4. Allowable design values for SIP panels with lumber splines are tabulated. Extrapolation for wall heights greater than tabulated values is not allowed.
5. SPF No.1/No.2 or better lumber splines.
6. Lumber splines to be full wall height.

EXTERIOR WALL DESIGN

SIP WALL PANEL with 2-PLY LUMBER SPLINE (cont.)

TABLE 7C

| Maximum Allowable Axial Load for 8.25" Panel (PLF) | | | | | | | | | | |
|--|------------------|-----------|------|------|------|------|------|------|------|------|
| Spline Type: 2-Ply Lumber Spline | | | | | | | | | | |
| Panel Height (feet) | Deflection Limit | Wind Load | | | | | | | | |
| | | 0 | 5 | 10 | 15 | 20 | 25 | 30 | 35 | 40 |
| 8 | L/360 | 4368 | 4286 | 4205 | 4123 | 4042 | 3960 | 3879 | 3797 | 3716 |
| | L/240 | 4368 | 4286 | 4205 | 4123 | 4042 | 3960 | 3879 | 3797 | 3716 |
| | L/180 | 4368 | 4286 | 4205 | 4123 | 4042 | 3960 | 3879 | 3797 | 3716 |
| 9 | L/360 | 4327 | 4225 | 4122 | 4020 | 3918 | 3816 | 3714 | 3611 | 3509 |
| | L/240 | 4327 | 4225 | 4122 | 4020 | 3918 | 3816 | 3714 | 3611 | 3509 |
| | L/180 | 4327 | 4225 | 4122 | 4020 | 3918 | 3816 | 3714 | 3611 | 3509 |
| 10 | L/360 | 4235 | 4112 | 3988 | 3865 | 3741 | 3618 | 3494 | 3371 | 3247 |
| | L/240 | 4235 | 4112 | 3988 | 3865 | 3741 | 3618 | 3494 | 3371 | 3247 |
| | L/180 | 4235 | 4112 | 3988 | 3865 | 3741 | 3618 | 3494 | 3371 | 3247 |
| 12 | L/360 | 4048 | 3878 | 3708 | 3538 | 3368 | 3198 | 3028 | 2858 | 2688 |
| | L/240 | 4048 | 3878 | 3708 | 3538 | 3368 | 3198 | 3028 | 2858 | 2688 |
| | L/180 | 4048 | 3878 | 3708 | 3538 | 3368 | 3198 | 3028 | 2858 | 2688 |

General Notes:

1. Table values are for linear pound per foot (lb/ft).
2. Loads Limited by Deflection or Ultimate Failure Load.
3. Tables values are for concentrated axial load with no eccentricity.
4. Allowable design values for SIP panels with lumber splines are tabulated. Extrapolation for wall heights greater than tabulated values is not allowed.
5. SPF No.1/No.2 or better lumber splines.
6. Lumber splines to be full wall height.

PWF FOUNDATION WALL DESIGN

PWF FOUNDATION WALL (8ft, 9ft & 10ft IN HEIGHT)

TABLE 8A

| Maximum Allowable Axial Load (PLF) | | | | | | | | | |
|------------------------------------|---------------------|------|------|---------------------|------|------|---------------------|------|------|
| Backfill Height (feet) | 8.25" (2x8) | | | 10.25" (2x10) | | | 12.25" (2x12) | | |
| | Panel Height (feet) | | | Panel Height (feet) | | | Panel Height (feet) | | |
| | 8 | 9 | 10 | 8 | 9 | 10 | 8 | 9 | 10 |
| 9 | | | | | | | | | |
| 8 | | | | | | | | 526 | |
| 7 | 1492 | 290 | | 2052 | 1026 | 127 | 2466 | 1609 | 729 |
| 6 | 2156 | 1393 | 344 | 2823 | 2104 | 1216 | 3160 | 2580 | 1709 |
| 5 | 2733 | 2323 | 1343 | 3493 | 3014 | 2114 | 3764 | 3398 | 2518 |
| 4 | 3196 | 3049 | 2109 | 4031 | 3724 | 2802 | 4248 | 4038 | 3137 |
| 3 | 3528 | 3558 | 2636 | 4416 | 4222 | 3276 | 4594 | 4486 | 3564 |
| 2 | 3727 | 3858 | 2942 | 4647 | 4515 | 3551 | 4802 | 4749 | 3811 |
| 1 | 3812 | 3983 | 3068 | 4746 | 4637 | 3665 | 4891 | 4860 | 3914 |

General Notes:

1. THIS TABLE IS FOR REFERENCE ONLY, THE FINAL DESIGN SHALL BE REVIEWED BY A LICENSED PROFESSIONAL ENGINEER.
2. Shaded values shown are for non-load bearing walls.
3. This table takes into account a surcharge load of 50psf.
4. The panels' deflection is limited to L/240.
5. The values in the table are for a standard load duration.
6. A linear axial+bending interaction relationship is taken into account.
7. The loads listed in the table are for loads with no eccentricity.
8. DEFLECTION CREEP FACTOR OF 1.5 FOR DEAD LOADS AS PER AWC DESIGN GUIDE.
9. A load duration factor of 0.9 is used to calculate strength capacity. Designer to select appropriate load duration factor based on the applicable loads.
10. It is recommended that the designer use the next thicker panel size if the applied load is very close to the allowable design load.

PWF FOUNDATION WALL DESIGN

PWF FOUNDATION WALL (4ft IN HEIGHT)

TABLE 8B

| Maximum Allowable Axial Load (PLF) | | | | | |
|------------------------------------|-----------------------|----------------------|------|------|------|
| Panel Width | Outside Backfill (ft) | Inside Backfill (ft) | | | |
| | | 4 | 3 | 2 | 1 |
| 4.5" | 4 | 1979 | 1058 | 364 | 53 |
| | 3 | 1979 | 1979 | 1278 | 948 |
| | 2 | 1979 | 1979 | 1979 | 1627 |
| | 1 | 1979 | 1979 | 1979 | 1979 |
| 6.5" | 4 | 2491 | 1691 | 1088 | 817 |
| | 3 | 2491 | 2491 | 1882 | 1595 |
| | 2 | 2491 | 2491 | 2491 | 2186 |
| | 1 | 2491 | 2491 | 2491 | 2491 |
| 8.5" | 4 | 2760 | 2042 | 1501 | 1258 |
| | 3 | 2760 | 2760 | 2214 | 1956 |
| | 2 | 2760 | 2760 | 2760 | 2486 |
| | 1 | 2760 | 2760 | 2760 | 2760 |

General Notes:

1. Deflection Limited to 0.25in
2. Design for permanent load. Reference CSA S406 2008
3. Table values are for standard load term.
4. Inside Backfill Linear axial+bending interaction relation is considered to allowable concentrated axial load with no eccentricity.
5. Load duration of 1.0 is considered for strength

SIP HEADER DESIGN

SIP HEADERS & LINTELS

TABLE 9A

| Maximum Allowable Axial Load (PLF) for 4.5" SIP Header/Lintel | | | |
|--|-----------------------|--------------------|------------------|
| Header Depth (inches) | Header Span (feet) | Maximum Load | |
| | | Uniform Load (plf) | Point Load (lbs) |
| 13 | 4 | 479 | 959 |
| | 6 | 320 | 959 |
| | 8 | 240 | 959 |
| 24 | 4 | 1050 | 2101 |
| | 6 | 700 | 2101 |
| | 8 | 525 | 2101 |
| 36 | 4 | 1807 | 3614 |
| | 6 | 1205 | 3614 |
| | 8 | 904 | 3614 |

TABLE 9B

| Maximum Allowable Axial Load (PLF) for 6.5" SIP Header/Lintel | | | |
|--|-----------------------|--------------------|------------------|
| Header Depth (inches) | Header Span (feet) | Maximum Load | |
| | | Uniform Load (plf) | Point Load (lbs) |
| 13 | 4 | 446 | 892 |
| | 6 | 297 | 892 |
| | 8 | 223 | 892 |
| 24 | 4 | 943 | 1886 |
| | 6 | 629 | 1886 |
| | 8 | 471 | 1886 |
| 36 | 4 | 1571 | 3141 |
| | 6 | 1047 | 3141 |
| | 8 | 785 | 3141 |

TABLE 9C

| Maximum Allowable Axial Load (PLF) for 8.25" SIP Header/Lintel | | | |
|---|-----------------------|--------------------|------------------|
| Header Depth (inches) | Header Span (feet) | Maximum Load | |
| | | Uniform Load (plf) | Point Load (lbs) |
| 13 | 4 | 432 | 864 |
| | 6 | 288 | 864 |
| | 8 | 216 | 864 |
| 24 | 4 | 898 | 1795 |
| | 6 | 598 | 1795 |
| | 8 | 449 | 1795 |
| 36 | 4 | 1471 | 2941 |
| | 6 | 980 | 2941 |
| | 8 | 735 | 2941 |

General Notes:

1. Designed loads as per NDS allowable stress design.
2. Table values are for standard load term.
3. Self weight must be included in load calculations.
4. This table is designed for common nail, 8d (D=0.131 in; L=2.5in) @ spacing 2" O.C.
5. Additional resistance reduction factor e.g. service condition, treatment factor must apply based on lumber condition.

WOOD FRAME HEADER DESIGN

WOOD FRAME HEADERS

TABLE 10

| Maximum Allowable Design Loads | | | | | | | |
|--------------------------------|-----------|--------------------|-----------------|--------------------|-----------------|--------------------|-----------------|
| Lumber Size | No. Plies | Span (ft) | | | | | |
| | | 4ft | | 6ft | | 8ft | |
| | | Uniform Load (plf) | Point Load (lb) | Uniform Load (plf) | Point Load (lb) | Uniform Load (plf) | Point Load (lb) |
| 2x6 | 1 | 358 | 1434 | 159 | 956 | 76 | 607 |
| | 2 | 717 | 2867 | 319 | 1912 | 152 | 1213 |
| | 3 | 1075 | 4301 | 478 | 2867 | 227 | 1820 |
| | 4 | 1434 | 5735 | 637 | 3823 | 303 | 2426 |
| 2x8 | 1 | 575 | 2300 | 256 | 1533 | 144 | 1150 |
| | 2 | 1150 | 4599 | 511 | 3066 | 287 | 2300 |
| | 3 | 1725 | 6899 | 767 | 4599 | 431 | 3449 |
| | 4 | 2300 | 9198 | 1022 | 6132 | 575 | 4599 |
| 2x10 | 1 | 858 | 3431 | 381 | 2288 | 214 | 1716 |
| | 2 | 1716 | 6863 | 763 | 4575 | 429 | 3431 |
| | 3 | 2574 | 10294 | 1144 | 6863 | 643 | 5147 |
| | 4 | 3431 | 13726 | 1525 | 9150 | 858 | 6863 |
| 2x12 | 1 | 1154 | 4614 | 513 | 3076 | 288 | 2307 |
| | 2 | 2307 | 9229 | 1025 | 6152 | 577 | 4614 |
| | 3 | 3461 | 13843 | 1538 | 9229 | 865 | 6921 |
| | 4 | 4614 | 18457 | 2051 | 12305 | 1154 | 9229 |

General Notes:

1. Designed loads as per NDS Allowable Stress Design for SPF No1/No2 or better grade lumber.
2. Table values are for allowable stress design. Use ASD load combination.
3. Self weight must be included in load calculations.
4. Headers to have a min. of 1.5" bearing at each end.
5. Additional resistance reduction factor e.g. service condition, treatment factor must apply based on lumber condition.
6. See Thermapan Installation Manual for header installation.

Product Warranty

Thermapan Structural Insulated Panels, Inc. warrants that its products will be free from manufacturing defects or errors in workmanship and material. In addition, the company warrants the adequacy of its design for the normal and expected life of the building, provided the product is correctly installed and used.

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